A stable and accurate algorithm for a generalized Kirchhoff-Love plate model

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Introduction

Kirchhoff-Love model [1]:

- Developed in 1888 by Love using assumptions proposed by Kirchhoff
- One of the most common dimensionally-reduced models of a thin linearly elastic plate
- Analytical solutions are available only to a limited number of cases with simple specifications [2]

Chladni's patterns [3]:

- Show the nodal lines, where no vertical displacements occurred, of the different natural modes of vibration
- Natural mode: a pattern of motion in which all parts of the system move sinusoidally with the same frequency and with a fixed phase relation
- Plate resonates at the natural frequencies

Formulation

Kirchhoff-Love theory's assumptions

- The plate is thin
- The displacements and rotations are small
- Transverse shear strains are neglected
- The transverse normal stress is negligible compared to the other stress components

Governing Equations: $\rho h \ddot{w}(t,x,y) = -\mathcal{K}w(t,x,y) - \mathcal{B}\dot{w}(t,x,y) + f(t,x,y)$

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Operators	Description	Parameters	Description
$\mathcal{K} = K_0 I - T\nabla^2 + D\nabla^4$	Time-invariant, symmetric	h	Constant thickness
	differential operator	ho	Density
$\mathcal{B} = K_1 I - T_1 \nabla^2$	Damping operator	K_0	Linear stiffness coefficient
$\nabla^2 = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2}$	Laplacian operator	T	Tension coefficient
		D	Bending stiffness
$\nabla^4 = \frac{\partial^4}{\partial x^4} + 2\frac{\partial^4}{\partial x^2 x y^2} + \frac{\partial^4}{\partial y^4}$	Biharmonic operator	ν	Poisson's ratio
Ox^{\perp} $Ox^{\perp}xy^{\perp}$ Oy^{\perp}		K_1	Linear damping coefficient
		T_1	Visco-elastic damping coefficient

Boundary & Initial Conditions

Boundary Conditions

We consider the following three common types of physical boundary conditions for a plate:

Clamped: w(t, x, y) = 0,w(t, x, y) = 0, $-D\left(\frac{\partial^2 w}{\partial n^2} + \nu \frac{\partial^2 w}{\partial t^2}\right)(t, x, y) = 0$ Simply Supported:

 $-D\left(\frac{\partial^2 w}{\partial n^2} + \nu \frac{\partial^2 w}{\partial t^2}\right)(t, x, y) = 0, \quad -D\frac{\partial}{\partial n} \left[\frac{\partial^2 w}{\partial n^2} + (2 - \nu) \frac{\partial^2 w}{\partial t^2}\right](t, x, y) = 0$ • Free:

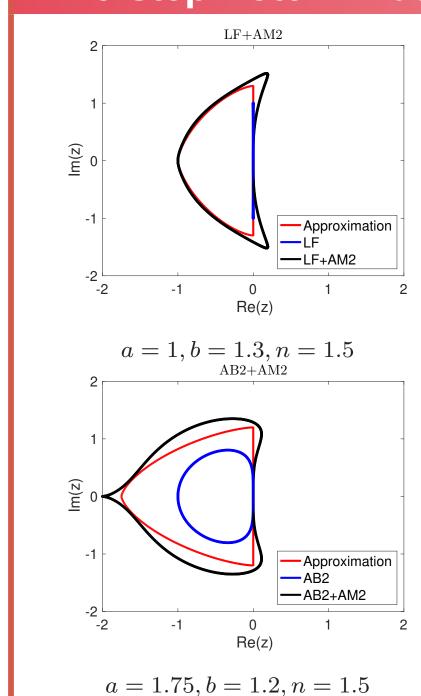
Initial Conditions: $w(0,x,y)=\alpha(x,y), \quad \dot{w}(0,x,y)=\beta(x,y),$ for given functions $\alpha(x,y)$ and $\beta(x,y)$

Numerical Method

Centered finite difference approximation for spatial discretization For time integration

- Explicit predictor-corrector time-stepping method:
- Predictor: Leapfrog (LF) or Adams-Bashforth (AB2); Corrector: Adams-Moulton (AM2)
- Implicit Newmark-Beta (NB) method: for $\beta = 1/4$ and $\gamma = 1/2$, the NB method is second order accurate and unconditionally stable

Time-step Determination



Region of absolute stability

Half super-ellipse to approximate the region of absolute stability:

$$\left|\frac{\Re(z)}{a}\right|^n + \left|\frac{\Im(z)}{b}\right|^n \le 1, \qquad \Re(z) \le 0,$$

where $\Re(z)$, $\Im(z)$ are real and imaginary parts of z, respectively. Stable time step: $\Delta t =$

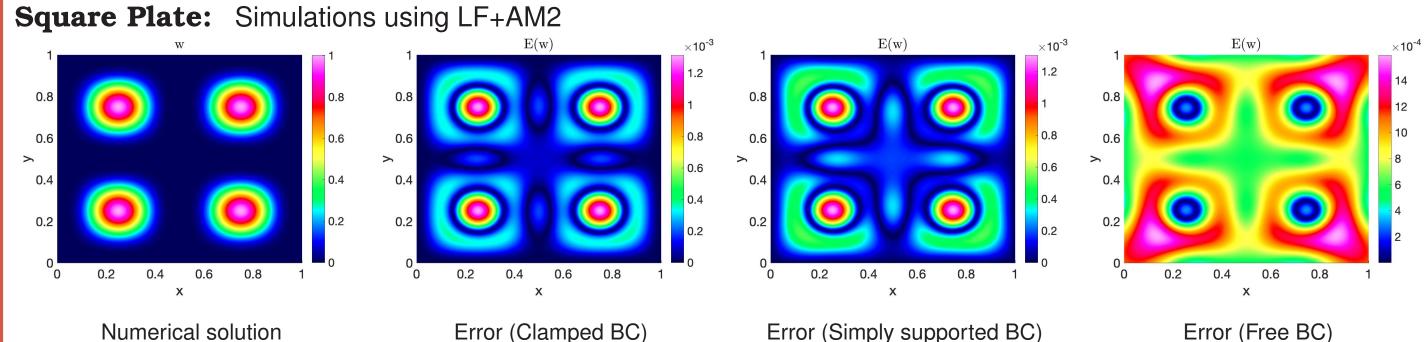
where $C_{\text{cfl}} \leq 1$ is the Courant-Friedrichs-Lewy (CFL) number and

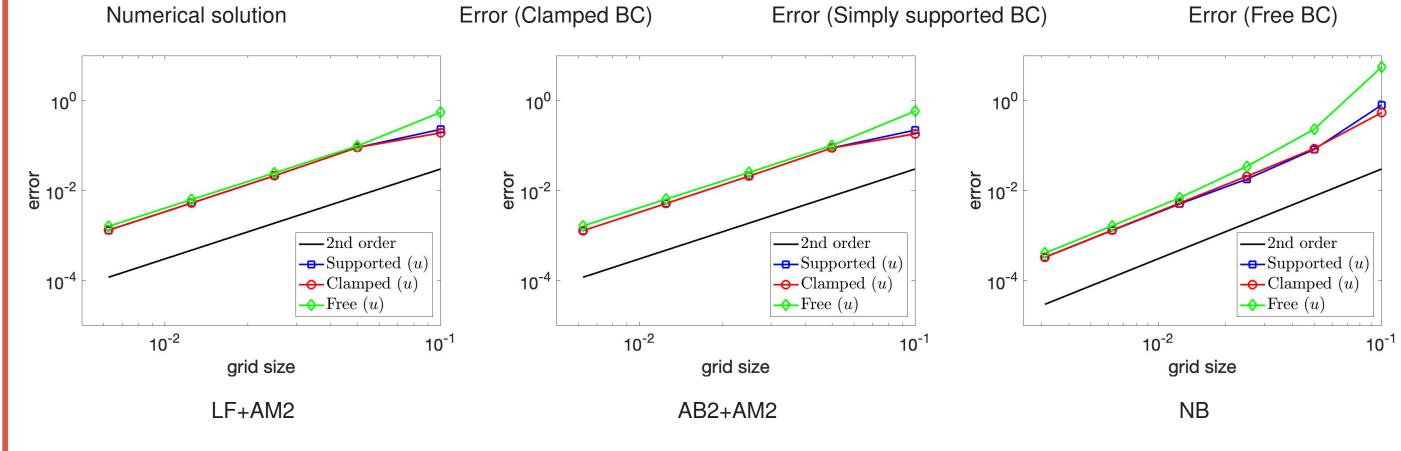
$$\hat{\lambda}_{M} = \begin{cases} -\frac{\hat{\mathcal{B}}_{M}}{2} \pm i \sqrt{\hat{\mathcal{K}}_{M} - \left(\frac{\hat{\mathcal{B}}_{M}}{2}\right)^{2}}, & \text{if } \left(\frac{\hat{\mathcal{B}}_{\omega}}{2}\right)^{2} - \hat{\mathcal{K}}_{\omega} < 0 \\ -\hat{\mathcal{B}}_{M}, & \text{if } \left(\frac{\hat{\mathcal{B}}_{\omega}}{2}\right)^{2} - \hat{\mathcal{K}}_{\omega} > 0 \end{cases}$$

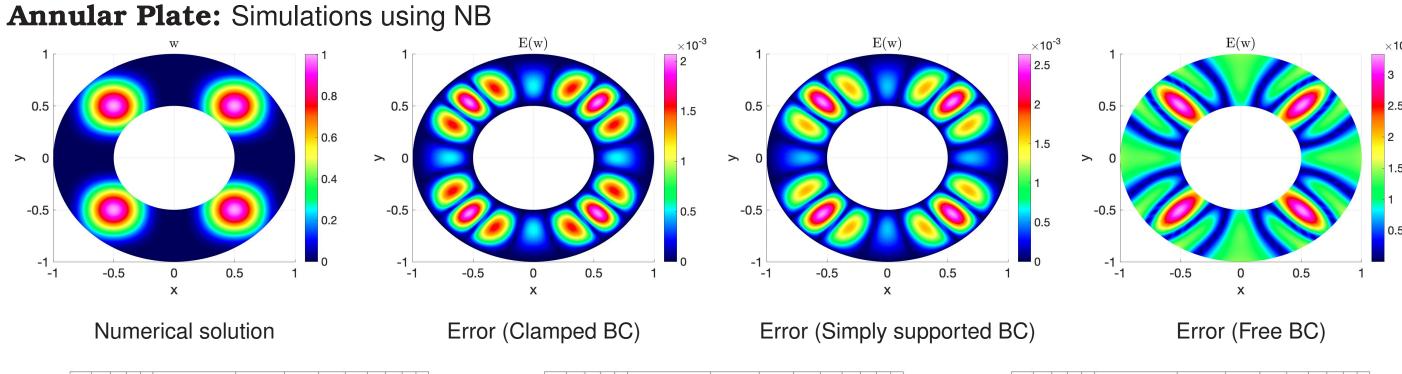
For NB: C_{cfl} can be taken as big as 100

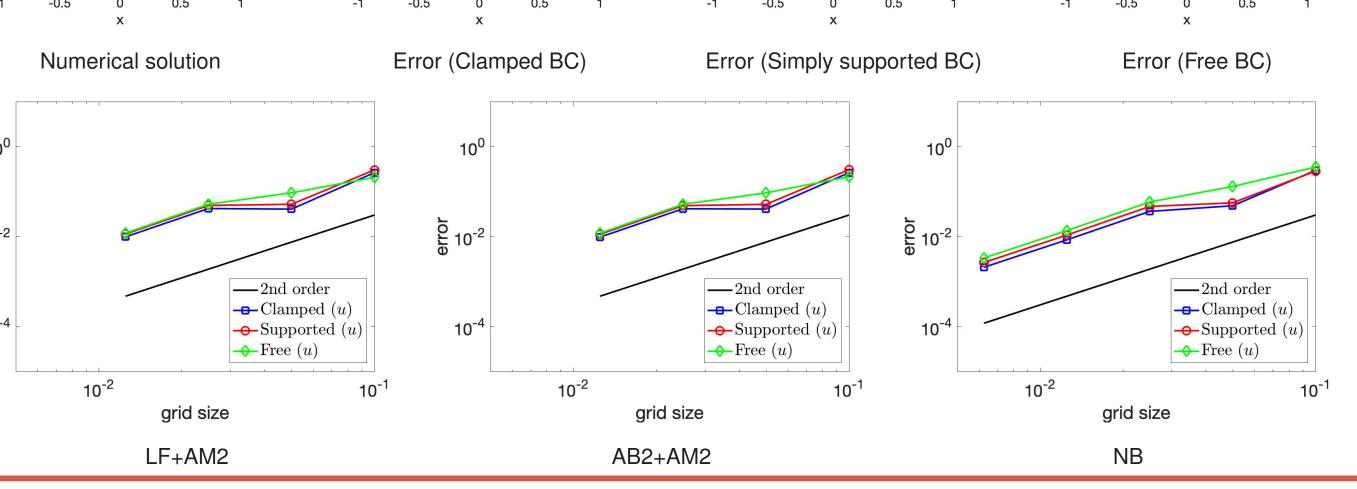
Results I: Method of Manufactured Solution

Manufactured solution: $w_e(t, x, y) = \sin^4(2\pi x) \sin^4(2\pi y) \cos(2\pi t)$ Material properties: $\rho h = 1, K_0 = 2, T = 1, D = 0.01, K_1 = 5, T_1 = 0.1, \nu = 0.1$ Forcing term: $f(t, x, y) = \rho h \frac{\partial^2 w_e}{\partial t^2} + K_0 w_e - T \Delta w_e + D \Delta^2 w_e + K_1 \frac{\partial w_e}{\partial t} - T_1 \Delta \frac{\partial w_e}{\partial t}$ Initial conditions: w(0,x,y) = 0, $\dot{w}(0,x,y) = 0$









Results II: Standing Waves and Nodal Lines

- Free vibration: f(t, x, y) = 0, $w(0, x, y) = \phi(x, y)$, $\dot{w}(0, x, y) = 0$
- Forced vibration: $f(t, x, y) = F_0 \cos(\Omega t) \delta_{(x-x_0, y-y_0)}, \quad w(0, x, y) = 0, \quad \dot{w}(0, x, y) = 0$

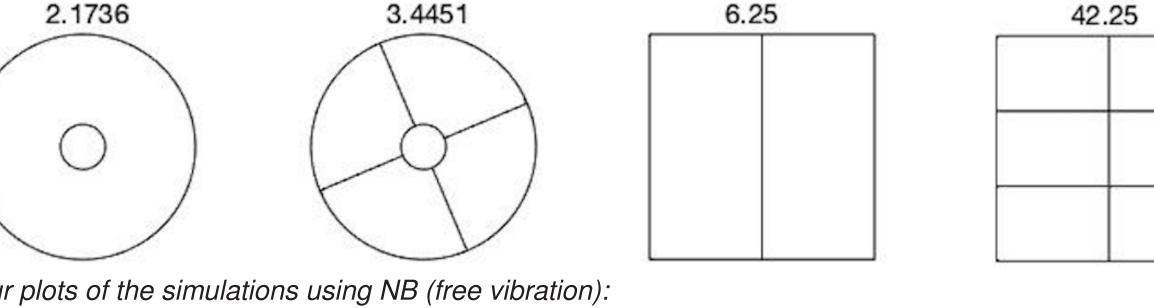
 Ω and $\phi(x,y)$: eigenvalue and eigenvector of the eigenvalue problem $\mathcal{K}\phi(x,y)=\lambda\phi(x,y)$

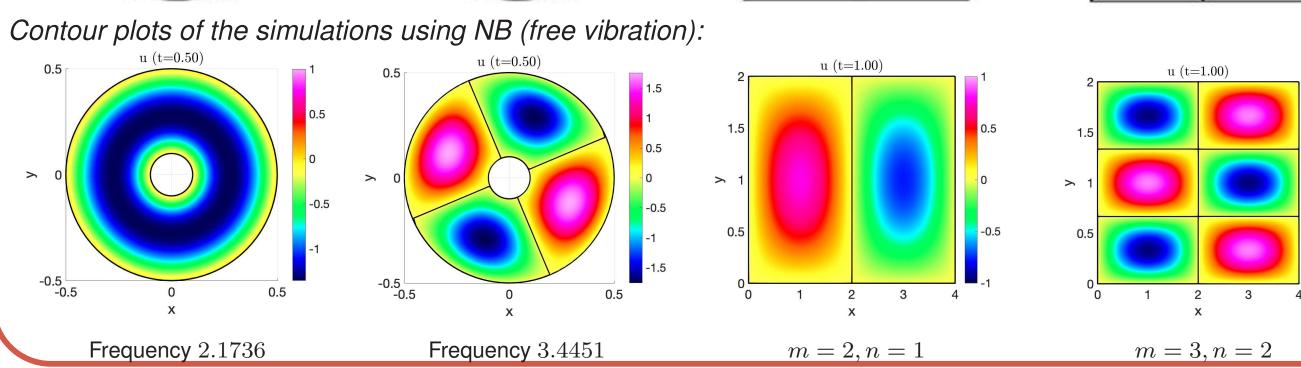
1. Simply Supported BC

Material properties: $\rho h = 1, K_0 = 0, T = 0, D = 2, K_1 = 0, T_1 = 0, \nu = 0.1$

(Rectangular) Eigenvector: $\phi(x,y) = \sin\frac{m\pi x}{a}\sin\frac{n\pi y}{b}$, eigenvalue: $f_{mn} = \left(\frac{m^2}{a^2} + \frac{n^2}{b^2}\right)\sqrt{\frac{\bar{D}\pi^2}{4\bar{\rho}\bar{b}}}, m, n = 1, 2, \dots$

Nodal line plots from the solution of the eigenvalue problem using the eigs function in MATLAB:



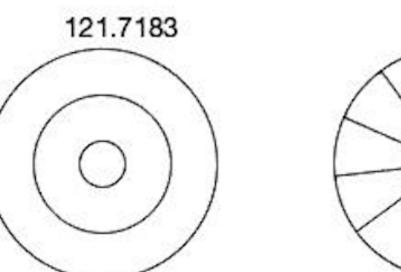


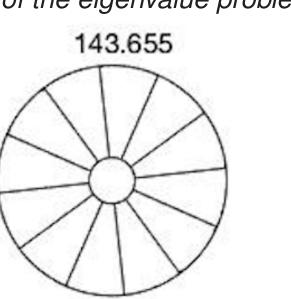
Results II: Standing Waves and Nodal Lines

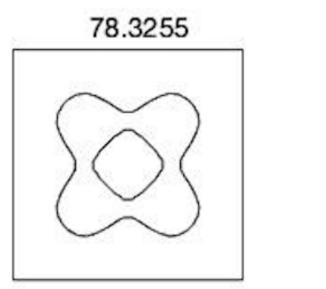
2. Clamped BC:

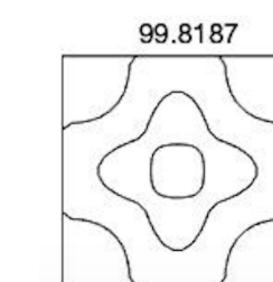
Material properties: $\rho h = 1, K_0 = 2, T = 1, D = 0.01, K_1 = 0, T_1 = 0, \nu = 0.1, F_0 = 10^8$

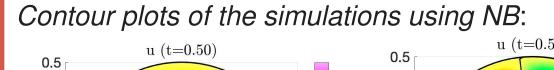
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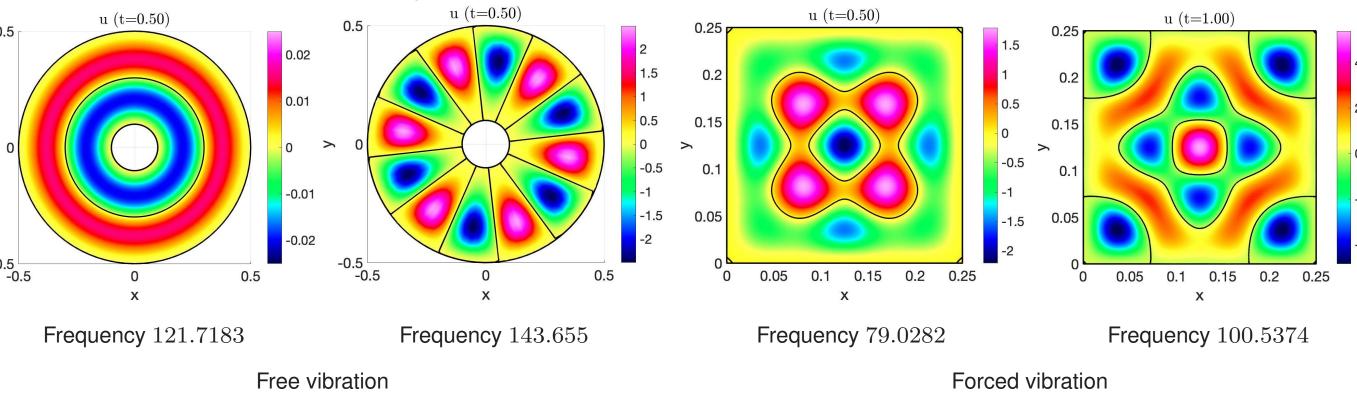








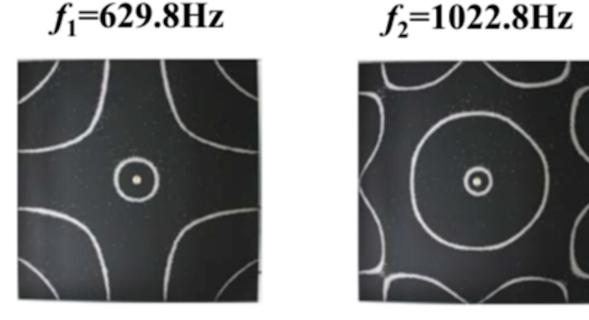


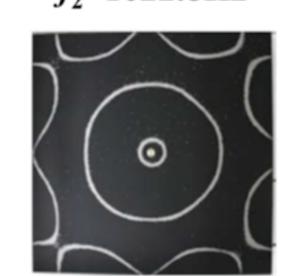


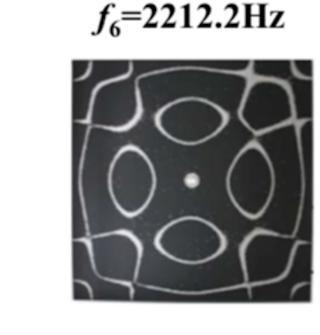
3. Free BC:

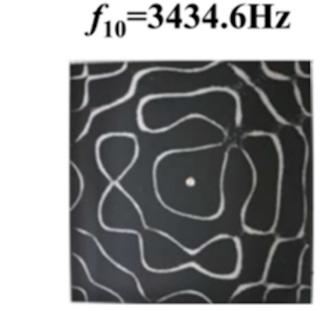
Material properties (Aluminum): $\rho = 2700, h = 0.001, K_0 = 0, T = 0, E = 69, K_1 = 0.1, T_1 = 5, \nu = 0.33, F_0 = 10^8$ The center of the plate is clamped.

Experimetal results [4]:

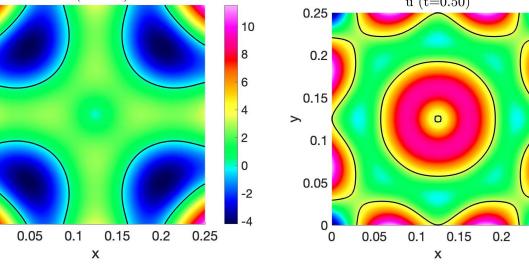


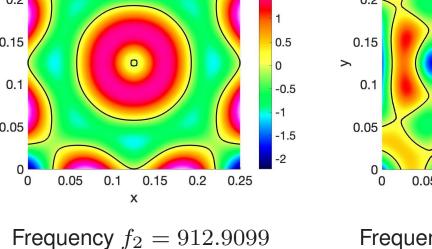


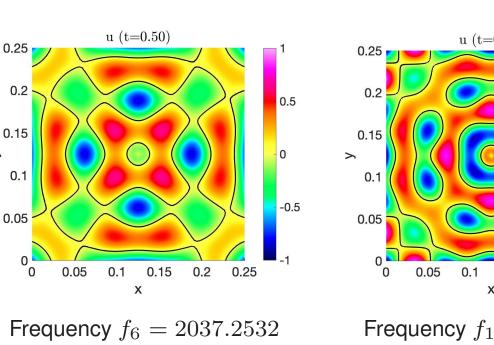




Simulated results using NB (forced vibration):





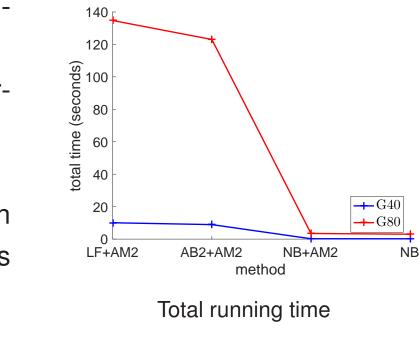


Frequency $f_{10} = 3159.832$

Conclusion

Frequency $f_1 = 560.5412$

- A sequence of benchmark problems with increasing complexity are considered to demonstrate the numerical properties of the algorithm
- Mesh refinement study, with the method of manufactured solutions, verifies the stability, accuracy and second order convergence
- NB is more time-efficient than the explicit predictor-corrector schemes
- Nodal lines, natural mode shapes and frequencies obtained through free and forced vibrations match the expected and experimental results



Future Directions

- Extend current research to more complicated geometries
- Couple the developed plate solver with an existing fluid solver to simulate more interesting fluid-structure interaction (FSI) problems, such as blood flow in an artery

References

- [1] A. E. H. Love, On the small free vibrations and deformations of elastic shells, Philosophical trans. of the Royal Society (London), 1888, Vol. serie A, No 17 p. 491 – 549.
- [2] Rudolph Szilard, *Theories and Applications of Plate Analysis*, Classical Numerical and Engineering Methods, 2004, Wiley, p. 62 - 127.
- [3] W. M. Pierce, Chladni Plate Figures, American Journal of Physics 19(7), 1951, DOI: 10.1119/1.1933030.
- [4] P. H. Tuan, C. P. Wen, P. Y. Chiang, Y. T. Yu,H. C. Liang,K. F. Huang, Y. F. ChenKML, *Exploring the resonant vibration of thin* plates: Reconstruction of Chladni patterns and determination of resonant wave numbers, The Journal of the Acoustical Society of America 137, 2113 (2015), doi: 10.1121/1.4916704